



Flow over Flexible Vegetated Bed: Evaluation of Analytical Models

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ABSTRACT

The development of vegetation in the river bed and in the banks can affect the hydrodynamic conditions and the flow behavior of a watercourse. This can increase the risk of flooding and sediment transport. Therefore, it is important to develop analytical approaches to predict the resistance caused by vegetation and model its effect on the flow. This is the objective of this work which investigates the ability of different analytical models to predict the vertical velocity profile as well as the resistance induced by flexible submerged vegetation in open channels. Then it is possible to select the appropriate model that will be applied in the real case of rivers. The model validation is determined after a comparison between the data measured in the different experiments carried out and those from literature. For dense vegetation, the role of the Reynolds number is emphasized in particular with a model using the Darcy-Brinkman equation in the canopy. With a simple permeability, this model is relevant to estimate friction. However, for larger Reynolds number, models based on the fully turbulent flow assumption provide better results.

Keywords: Analytical models; Experiments; Flexible vegetation; Open channel; Roughness.

1. INTRODUCTION

Vegetation in rivers and floodplains occurs in different forms; it can be flexible or rigid and submerged or emerged in the flows. It plays an important role in the flow patterns of many streams and rivers and can increase the risk of flooding. In fact, understanding vegetation flows is necessary to control flooding and the river ecosystem (Wu and He, 2009; Liu and Shen, 2008). The influence of vegetation on the water routing during flash flood events are also crucial because they are determinant for forecasting and alert (Douinot *et al.*, 2017).

Indeed, the double averaged approach (Nikora *et al.* 2001) used to study flow above vegetation is particularly adapted for routing process where vegetation is dense, when steep slopes involve uniform flow and with a weak submergence. In the last decade, much research has been devoted to understanding vegetation flow characteristics using laboratory experiments with natural or artificial vegetation (Poggi *et al.*, 2004; Ghisalberti and Nepf, 2006; Le Bouteiller and Venditti, 2015). In fact, most of the developed relationships have adopted a two-layer approach. This method is based on dividing the flow domain into two-layer (Klopstra *et al.*, 1997; Defina and Bixio, 2005; Murphy and Nepf, 2007).

The first layer through the vegetation is called "vegetation or canopy layer", the second layer above is called "upper layer". The logarithmic flow velocity profile is adopted to solve the velocity above the vegetation, and the equation of momentum in the vegetated layer.

For non-vegetated flow, the vertical distribution of velocity is directly related to the shear stress of the bed, whereas for a vegetated flow, it is linked to the vegetation drag because the roughness of the vegetation is much greater than the roughness of the riverbed. Here, this study compares different analytical models based on the two-layers approach to analyze their performances. We focus on models which can predict the vertical flow velocity profiles through the submerged flexible vegetation. In Cassan *et al.* (2017), the computation of the velocity profile allows calculating the discharge for a large range of flow over rigid macro-roughness. The results have shown a quite good applicability for steep flow over gravels and rocks. For flexible canopy, the physical process could be different (monami wave, interaction stem-flow, stem vibration) (Marjoribanks *et al.*, 2017; Kucukali and Hassinger, 2018) and it is proposed to assess the same methodology to obtain discharge. In previous studies Morri *et al.* (2015) and Katul *et al.* (2011) have already compared the flow resistance provided by models where only one averaged velocity is

given for each layer. Then we choose to evaluate the Huthoff model which seems to be the most pertinent for flexible vegetation (Morri *et al.*, 2015) and three models using the double averaged approach (Nikora *et al.*, 2001).

A model which provides the velocity distribution can appear more complex, but it can be used to understand pertinent phenomena involved in the hydraulic resistance (King *et al.*, 2012) and transport phenomena. The models are compared for experiments from literature with flexible vegetation. Several studies had been already used (Poggi *et al.* 2009; Katul *et al.* 2011; Cassan and Laurens 2016), but we added to them more recent measurements (Le Bouteiller and Venditti, 2015).

Moreover, additional experiments were carried out in the case of dense canopy. These specific conditions were tested in order to understand difference between models based on Darcy-Brinkman equation (Rubol *et al.*, 2018) and those with a turbulent drag force (Klopstra *et al.* 1997; Meijer and Velzen 1999; Cassan and Laurens 2016).

2. MATERIALS AND METHODS

The analytical and semi-analytical models evaluated in the paper are here presented. They consider a momentum balance in a uniform flow and vegetation. The vegetation is an arrangement of obstacle on which is applied a drag force. An additional stress is added representing the turbulent term in the flow or/and in the bed boundary layer (Huthoff *et al.*, 2007). The drag force is expressed with a drag coefficient C_d and the frontal area by unit width a (m^{-1}). The frontal area can be obtained with the number of stem per square meter, m , and the stem diameter D which are usually given. Except the Huthoff model, the Meijer, Cassan and Rubol models are based on spatial and temporal averaged concept (Nikora *et al.*, 2001), and the double averaged velocity at a given vertical position is denoted u . The deflected height stem is denoted h_p and the total water depth H . As consequence the momentum balance in the upper layer leads to define the shear velocity as $u_* = \sqrt{gS(H - h_p)}$ where g is the gravitational constant ($9.81 m^2/s$) and S the friction slope which is equal to the bed slope in the experiments considered. Above vegetation, a logarithmic profile is often assumed for velocity distribution (Klopstra *et al.* 1997; Defina and Bixio 2005; Meijer and Velzen 1999) (Eq. (1)).

$$\frac{u}{u_*} = \frac{1}{\kappa} \ln\left(\frac{z-d}{z_0}\right) \quad (1)$$

where κ is the von Karman constant, d is the distance between the top of the vegetation and the virtual bed of surface layer, z_0 is the height of the roughness.

2.1. Huthoff *et al.*, 2007

To determine the expression of velocity, Huthoff *et al.* (2007) applied the two-layer approach, and the flow into and out of vegetation is treated separately.

The averaged velocity within the vegetation layer U_r is obtained by solving the momentum balance and by the Strikler law for friction on bed (Eq. (2)).

$$U_r = \sqrt{\frac{2gSb}{1 + \frac{b}{32h_p} \left(\frac{K_S}{H}\right)^{1/3}}} \sqrt{\frac{H}{h_p}} \quad (2)$$

where $b = 1/C_d m D$ is a drag length and m is the number of stems per unit area.

In the upper layer, Huthoff *et al.* (2007) used the Boussinesq hypothesis to describe shear stress, and considered the dissipation at the top of the canopy. It is possible to scale the velocity above the canopy as a function of a turbulent length scale (Eq. (3)). By comparison with other possible length scales, the chosen value of l is given by the distance between stem s . Finally, in the upper layer, the averaged velocity expression is given by the following equation (Eq. (3)):

$$U_s = U_r \sqrt{\frac{h_p}{H} \left(\frac{H-h_p}{s}\right)^{2/3} \left(1 - \left(\frac{H}{h_p}\right)^{-5}\right)} \quad (3)$$

2.2. Rubol *et al.* 2018

This model solves the Darcy-Brinkman equation in the canopy layer to compute the log-law parameters in the upper layer. The main novelty and difference with following models, is the drag force which depends linearly on the permeability K and the velocity u . For several experiments the value of K has been calibrated but it seems difficult to establish a general correlation. According to Battiato *et al.* (2014) for rigid stems, the permeability is expressed as proposed by Happel (1959) for laminar viscous flow through a regular array of cylinders (Eq. (4)):

$$K = R_1^2 \frac{1}{8} \left[-\ln(1 - \phi) - \frac{(1-\phi)^{-2}-1}{(1-\phi)^{-2}+1} \right] \quad (4)$$

Where $R_1 = 1/(2a)$, $\phi = 1 - R_0^2/R_1^2$ and $R_0 = D/2$.

Knowing the K value, it is possible to compute a complete velocity vertical distribution. The total discharge is obtained from direct integration of the velocity (see Eq. (6) of Rubol *et al.* (2018)).

As the friction is concerned, a good correlation is observed between the Darcy friction factor $f = 8gSH/U_b^2$ (U_b is the bulk velocity) and the Reynolds based on the shear velocity $Re_* = U_b H / \kappa u_* h_p$. This correlation is suggested to be a universal scaling. For emergent vegetation, Cheng and Nguyen (2011) had also found that for low Reynolds, a similar trend can be observed. Here, the simpler correlation proposed is considered (Eq. (5)). This relationship is another mean to calculate the discharge, and it will be analyzed further in the results part.

$$f = \frac{1}{Re_*^{1.38}} = \left(\frac{\kappa h_p \sqrt{gS(H-h_p)}}{U_b H} \right)^{1.38} \quad (5)$$

2.3. Klopstra 1997

The first formulation of the vertical velocity profile in an aquatic canopy was done by Klopstra *et al.* (1997). The momentum equation within the canopy



Fig. 1. Picture of the vegetation used for experiments S1 (left), S2 (center) and S3 (right).

is solved for a uniform and steady flow over vegetation. The analytical solution for velocity is computed considering the following turbulent shear stress τ :

$$\tau = \rho \alpha u \frac{du}{dz} \quad (6)$$

Where α (m) is a length scale. The velocity within the canopy is provided by the integration of the double averaged momentum balance. The boundary conditions for this integration in the vertical direction are no turbulence at the bed (Eq. (7)) and a shear stress at the canopy given by ρu_*^2 . The continuity at the top of canopy gives the values of the parameter d and z_0 .

$$u_0 = \sqrt{\frac{2gS}{\alpha c_d}} \quad (7)$$

The correlation given by Klopstra *et al.* (1997) of α is:

$$\frac{\alpha}{h_p} = 0.0793 h_p \ln\left(\frac{H}{h_p}\right) - 0.0009 \quad (8)$$

The influence of H could be linked to the penetration depth or a parameter of the mixing layer usually used in a two-layer model (Konings *et al.* 2012; Nepf, 2012; Nikora *et al.* 2013; Katul *et al.* 2011; Carollo *et al.* 2002).

This model will not be directly evaluated because the Eq. (8) was obtained with too few experiments. However, the following models reuse the same assumption and flow description.

2.4. Meijer, 1999/ Defina and Boixo, 2005

In the continuation of the Klopstra work, Meijer and Velzen (1999) carried out experiments to improve correlation on α for real vegetation. The calibration had revealed that the ratio H/h_p is also necessary to well understand the results. They established a new turbulent closure (Eq. (9)):

$$\frac{\alpha}{h_p} = 0.0144 \sqrt{\frac{H}{h_p}} \quad (9)$$

2.5. Cassan and Laurens, 2016

With the same concept, the length scale of the turbulent closure is calibrated with experiment series with rigid stems reported by Poggi *et al.* (2009) and experiments with cylindrical macro-

roughness. The vegetation density is expressed as a ratio of area in a horizontal plane, $C = mD^2$. The length is scaled considering the geometrical distance, s , between stem or h_p the stem height. This choice was made to integrate flow description in Huthoff *et al.* (2007). To consider the influence of the upper layer, the length scale is related to the upper flow and it is coupled to the lower layer flow by the turbulent viscosity continuity. This assumption, regarding the turbulent properties at the top of canopy has provided good results for rigid vegetation and low submergence. Therefore, the Eq. (9) is substituted by the Eq. (10).

$$l = \min(0.15h_p, s) \quad (10)$$

The drag coefficient is also modified to take into account the ratio between h_p and D and the flow interaction with the bed. These corrections were necessary to ensure the continuity between the emergent and submerged vegetated flows. Finally, the vertical velocity distribution is given by the Eq. (11) for the lower layer.

$$u(\tilde{z}) = u_0 \sqrt{\beta \left(\frac{H}{h_p} - 1\right) \frac{\sinh(\beta \tilde{z})}{\cosh(\beta)}} + 1 \quad (11)$$

With $\tilde{z} = \frac{h_p}{\alpha} \frac{C_d C h_p / D}{1 - \pi / 4 C}$, $\tilde{z} = z/h_p$ is the dimensionless vertical position.

2.6. Experiments

The first experiments concerned a flexible vegetated bottom at the INAT (National Institute of Agronomy of Tunisia) laboratory in a rectangular channel 5 m long, and 0.075 m wide and 0.15 m deep. The aim is to compare the experimental results with the analytical models for very dense canopy. On the bottom, initially smooth, we glued, in the longitudinal direction of the flow a vegetation cover that has 40 mm as height of fibers distributed in the center of the channel as indicated in Fig. 1. The number of stems is obtained by counting them of an area of 5 cm by 5 cm. The water depth is measured by analyzing side views from a camera (640*230 pixels with 1 pixel=0.625 mm).

The second series of experiments were conducted in a flume 5.75 m long and 0.29 m wide (Montpellier Supagro, France). The bed was covered with artificial flexible vegetation made with thin circular

Table 1 Experimental results for 3 types of dense vegetation. For S2 and S3, *D* corresponds to the equivalent diameter due to several stems

exp	Q (m ³ /s)	H (m)	h _p (m)	S (m/m)	α (m ⁻¹)	m (stem/m ²)	D (m)	C
S1-1	9.32E-05	0.047	0.04	0.03	55	86000	0.001	0.06
S1-2	1.53E-04	0.055	0.04	0.03	55	86000	0.001	0.06
S1-3	2.09E-04	0.058	0.04	0.03	55	86000	0.001	0.06
S1-4	3.24E-04	0.059	0.04	0.03	55	86000	0.001	0.06
S1-5	4.08E-04	0.0641	0.04	0.03	55	86000	0.001	0.06
S1-6	7.08E-04	0.07	0.04	0.03	55	86000	0.001	0.06
S1-7	7E-4	0.07	0.04	0.03	55	86000	0.001	0.06
S1-8	6E-4	0.06	0.04	0.03	55	86000	0.001	0.06
S1-9	2E-4	0.05	0.04	0.03	55	86000	0.001	0.06
S2-1	0.005	0.132	0.07	0.003	25.6	6400	0.004	0.102
S2-2	0.01	0.165	0.07	0.003	25.6	6400	0.004	0.102
S2-3	0.015	0.2	0.07	0.003	25.6	6400	0.004	0.102
S2-4	0.005	0.12	0.07	0.005	25.6	6400	0.004	0.102
S2-5	0.01	0.155	0.07	0.005	25.6	6400	0.004	0.102
S2-6	0.015	0.175	0.07	0.005	25.6	6400	0.004	0.102
S3-1	0.005	0.12	0.07	0.003	14.4	3600	0.004	0.058
S3-2	0.01	0.145	0.07	0.003	14.4	3600	0.004	0.058
S3-3	0.015	0.165	0.07	0.003	14.4	3600	0.004	0.058
S3-4	0.005	0.115	0.07	0.005	14.4	3600	0.004	0.058
S3-5	0.01	0.14	0.07	0.005	14.4	3600	0.004	0.058
S3-6	0.015	0.16	0.07	0.005	14.4	3600	0.004	0.058
S3-7	0.005	0.105	0.07	0.01	14.4	3600	0.004	0.058
S3-8	0.01	0.14	0.07	0.01	14.4	3600	0.004	0.058
S3-9	0.015	0.18	0.07	0.01	14.4	3600	0.004	0.058

cylinders of 0.8 mm diameter. A dozen of cylinders were gathered and stuck on a PVC blade at the same position. Then the diameter of a stem is 4 mm. These basic vegetation cylinders were set in a staggered arrangement with 2 different distances between stems corresponding to 2 different densities. The cylinder height is equal to 7 cm (Fig. 1). The spatial density, which is equal to $mD = C/D$, remains almost constant in the vertical direction although near the canopy cylinders are no more contiguous.

The flume slope could be adjusted from 0 to 3 %. A weir at the upstream end allows fixing different water depths. The flow discharge was measured with an electromagnetic flowmeter with an uncertainty lower than 1%.

Velocity profiles above the vegetation is obtained by Acoustic Doppler velocimetry with an micro-ADV Nortek Vectrino+ with a sample sampling rate equal to 25 Hz. The vegetation is assumed to be dense enough to cause a velocity profile independent of the lateral position relatively to the arrangement. The velocity measurements were performed for experiments S3-2, S3-3, S3-5 and S3-6.

3. RESULTS AND DISCUSSION

3.1. Velocity Profiles

The velocity distribution calculation with the models from Defina and Bixio (2005), Cassan and Laurens (2016) and Rubol *et al.* (2018) had been validated for several experiments. But the comparison for a dense case is difficult because of the measurement within the canopy. In addition of the present velocity measurements (Fig. 2), the experiment from Le Bouteiller and Venditti (2015) for dense vegetation which has never been compared to double averaged model, is particularly adapted because of their plant densities and simple canopy configuration. The Fig. 3 presents the velocity profiles for $C_d = 1$ where Re_s is 4200, 4600 and 3300. Similarly, the experiments with flexible vegetation and various densities could be used (Kubrak *et al.*, 2008) to precise the role of velocity within the canopy.

The Huthoff's model appears to be the less pertinent to compute accurate discharge, the calibration of the model was performed for sparser vegetation which could explain the discrepancies. However for the S3

experiments, the gap with experiments is reduced. Then a new calibration could be studied to keep the simplicity of the model but enlarging the range of applicability.

The Meijer and Cassan models are likely to better reproduce the velocity above the canopy and then the discharge. The flexible vegetation can be considered as rigid using the deflected height. The velocity computed with the Rubol model can differs greatly from experiments, in particular within the canopy. But the experiments chosen may be too sparse as regard of the validity range of the Darcy-Brinckman equation. This issue appears clearly by analysing the Kubrak's experiments where a good agreement is observed for the denser canopy (Fig. 4) but the difference increases when the density decreases. For the two first curves ($a=9 \text{ m}^{-1}$), the model provides a consistent discharge whereas for other experiments ($a=2.25 \text{ m}^{-1}$) the velocity is largely over estimated. Moreover, it must be kept in mind that the Rubol model could be improved by a better estimation of the permeability.

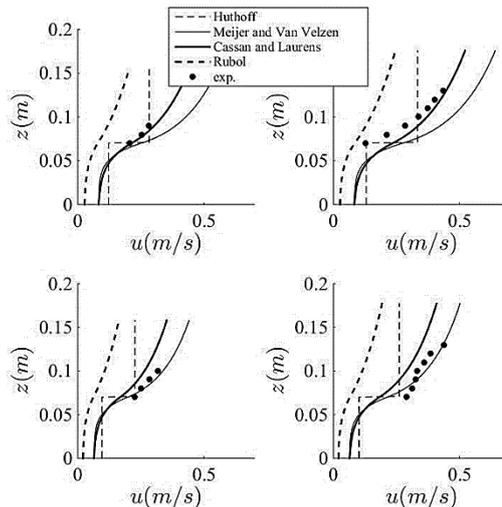


Fig. 2. Comparison of velocity profile of experiments S3-5 (a), S3-6 (b), S3-2 (c) and S3-3 (d) and the 4 models.

3.2. Friction Coefficient

For all models, the hydraulic resistance is described with a drag coefficient which expresses the drag force as a function of either the bulk velocity (C_{db}) (Rubol *et al.* 2018) or the velocity in the canopy (C_d) (other models).

With the first method in a steady uniform flow, the momentum equation on the water volume in the canopy is given by the equilibrium between shear stress at the canopy, the drag force and the water weight if the shear stress on bed is neglected. For the water volume around one stem, this consideration can be written as follows:

$$\frac{1}{2} C_{db} D h_p U_b^2 = \frac{u_*^2}{m} + \frac{g s h_p}{m} \quad (12)$$

Considering $u_* = \sqrt{gS(H - h_p)}$, the second term

can be written $u_*^2/m(1+1/(H/h_p-1))$. Rearranging Eq. (12), one obtains :

$$C_{db} = \left(\frac{1}{Re_*}\right)^2 \left(\frac{(H/h_p)^3}{a k h_p (H/h_p - 1)}\right)^2 \quad (13)$$

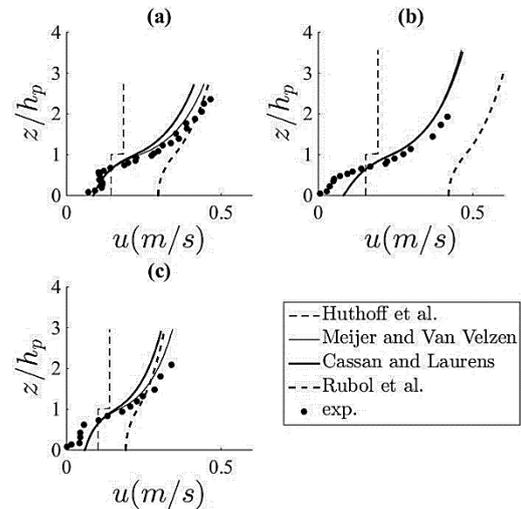


Fig. 3. Comparison of velocity profile of (Le Bouteiller and Venditti, 2015) experiments and the 4 models. $Re_* = 4200$ (a), 4600 (b) and 3300 (c).

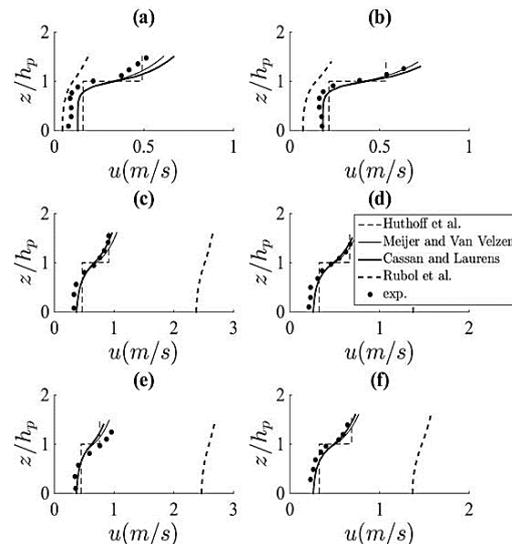


Fig. 4. Comparison of velocity profiles of (Kubrak *et al.*, 2008) experiments and the 4 models. The figures correspond respectively to the experiments 1.1.3 (a), 1.2.1 (b), 2.2.1 (c), 3.1.1 (d), 3.2.1 (e), 4.1.1 (f), 4.2.1 (g).

Usually the experiments with flexible vegetation are performed for a limited range of ratio H/h_p (between 1 and 4) and density ($a h_p$) between 1 and 10 m^{-1} . On the other hand, the variations of Re_* are quite larger, therefore it was effectively observed that C_{dB} decreases with the Reynolds number Re_* (Fig. 5) (Wilson, 2007) corresponding to the first term in parenthesis in the Eq. (13).

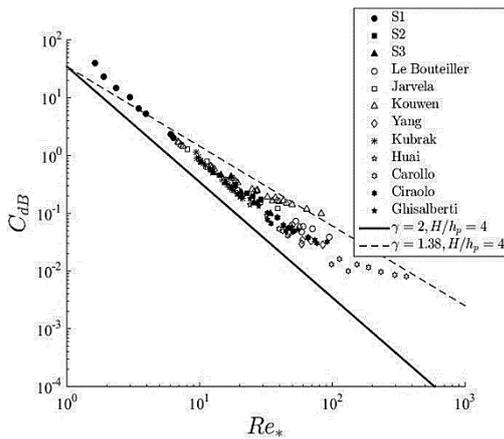


Fig. 5. Drag coefficient based on the bulk velocity as a function of Re_* ($ah_p = 3$ in Eq. (13)).

The experimental results from Rubol *et al.* (2018) shows that C_{dB} scale as $Re_*^{-\gamma}$ with $\gamma=1.38$. However, these measurements are significant only if the term $\left(\frac{(H/h_p)^3}{akh_p(H/h_p-1)}\right)^2$ is almost constant, otherwise the Eq. (13) could be finally another expression of the momentum balance with a constant friction coefficient. Formula using a Reynolds number based on the bulk velocity and molecular viscosity should be more relevant.

For the experiments from the present study, from Le Bouteiller and Venditti (2015) and those reported by Poggi *et al.* (2009), the general trend of C_{dB} is actually given by Re_*^{-2} for experiments with artificial stripes whereas the $\gamma=1.38$ agrees with the experiments with real vegetation (Carollo *et al.* 2002; Ciralo and Ferreri 2007). From Fig. 5, it can be stated that the experimental correlation from Rubol *et al.* (2018) is enough accurate for real dense vegetation. It also could be the evidence that the friction in the canopy does not depend on the square velocity ($\gamma \neq 2$). In other words, the viscous term or stem vibration on drag can be significant. However, it seems that the variation of C_{dB} from experiments could be also explained by a variation of H/h_p . As it is not possible to discriminate between the two explanations, this method is not studied further, and we focused on the velocity integration profile.

For experiments with artificial stems, the second method (with C_d) was developed to link the hydraulic resistance to fluid mechanical process in the canopy. As explained above, the main challenge is to well model the turbulence in the mixing layer at the top of the canopy because it is responsible for the velocity profile in the upper layer. For dense canopy, the larger part of the discharge flows in the upper layer, then it is crucial to well described the velocity profile. The comparison between computed discharges (Q_c) with the 4 models is drawn on the Fig. 6. The Huthoff model shows a good agreement for a large number of experiments and it is less efficient than others even if it is simpler. The difference occurs mainly for the denser canopies:

present experiments and those of (Carollo *et al.* 2002; Ciralo and Ferreri 2007). For experiments with real vegetation (Carollo *et al.* 2002; Ciralo and Ferreri 2007), the model performance is generally worse than for the experiments with stripes. It could be easily explained by the fact that C_d is assumed to be equal to 1 whereas it should integrate a Reynolds or a shape dependence due to leaves. For the Meijer model a discrepancy is noticed for the present's experiments (S1). The model from Cassan and Laurens (2016) improves slightly the Meijer's one by reducing the computed discharge thanks to a new formulation of the drag coefficient and turbulent length scale. Although the permeability K is not calibrated, the most efficient model is the Rubol's model which provides a very good agreement for the majority of experiments. However for some series (Kubrak *et al.* 2008; Yang and Choi 2009), the measured discharges differ from the computed ones.

To understand the reason of the discrepancies, the error between the computed and measured discharge is depicted on the Fig. 7 as a function of the Reynolds number based on the molecular viscosity $Re=UH/v$. As expected, the Rubol's model reproduces satisfactorily the discharge for the lower Reynolds number. The other models become pertinent only for $Re>20000$. The averaged accuracy could be estimated to 30 %. For the low Re , it could be stated that the model provides a better discharge prediction because it is the only one which can reproduce the increase of the drag coefficient for low Reynold flow. At high Re number the efficiency of this model decreases and those developed for fully turbulent flow and sparser canopy become more pertinent.

4 CONCLUSION

A description of the hydraulic resistance due to a flexible and dense vegetation is necessary to calculate the flows in the natural cases. The analytical models studied can give an estimation of the discharge. The role of the viscous term in the dense canopy has been emphasized since the model based on the Darcy-Brinckman equation provides performant result. Indeed, the most accurate model for friction prediction is the one of Rubol *et al.* (2018) which already integrates a low Reynolds number into the resolved physical law. This model depends on the canopy permeability and its formulation from Happel (1959) gives a relevant approximation similarly than study on rigid vegetation (Battiato *et al.* 2014). Further experimental studies are needed to improve calibration of this model, because measurements of permeability vegetation may be difficult in the field. For a higher Reynolds number flow, the analytical models (Meijer and Velzen, 1999; Cassan and Laurens, 2016) allows computing a vertical velocity distribution and they are relevant for replicating experimental results over a wide range of hydraulic conditions. Finally, it may be interesting to have later relevant models that will be applied for studies

with an intermediate Reynolds number.

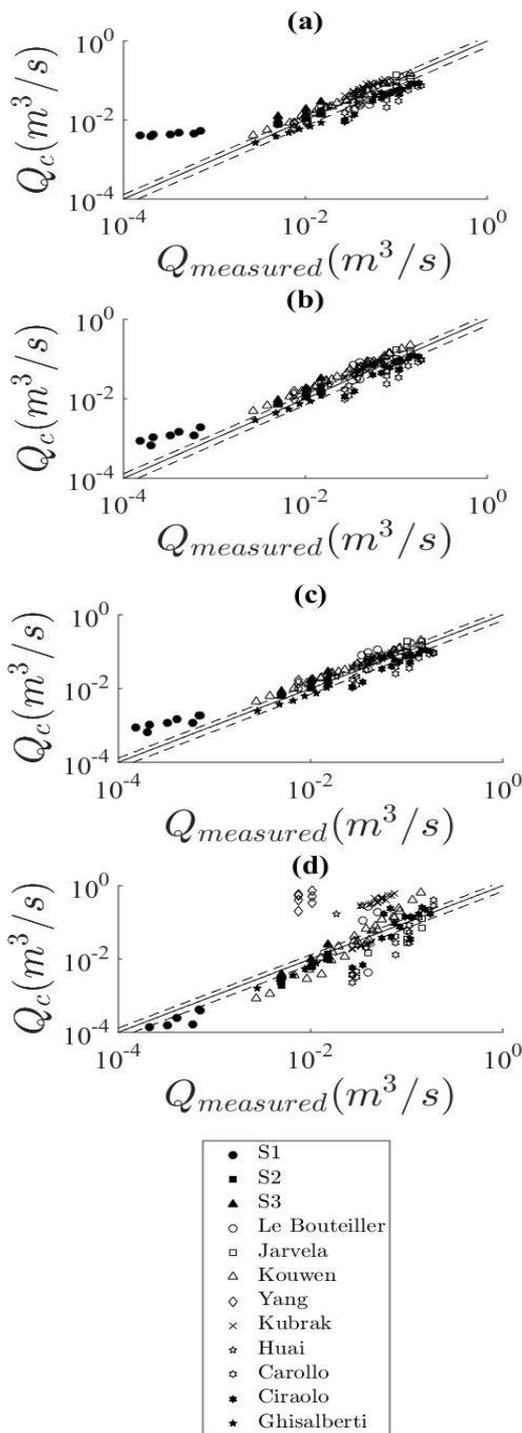


Fig. 6. Comparison of the total discharge over vegetated bed for the model of Huthoff (a), the Meijer and Van Velzen (b), Cassan and Laurens (c), and Rubol *et al.* (d) ($C_d = 1$ for all experiments). Experiments from (Le Bouteiller and Venditti, 2015; Jarvela, 2005; Kouwen and Unny, 1969; Yang and Choi, 2009; Kubrak *et al.*, 2008; Huai *et al.*, 2009; Carollo, Ferro, Termini, 2002; Ciralo, Ferreri, 2007; Ghisalberti and Nepf, 2006). Dash lines represents a deviation of 30 %.

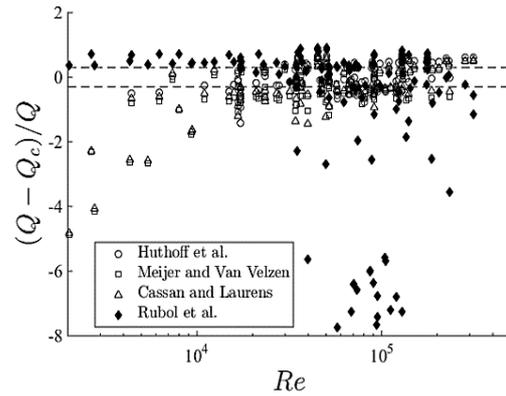


Fig. 7. Error between the 4 models and experiments from literature (Le Bouteiller and Venditti, 2015; Jarvela, 2005; Kouwen and Unny, 1969; Yang and Choi, 2009; Kubrak *et al.*, 2008; Huai *et al.*, 2009; Carollo, Ferro, Termini, 2002; Ciralo, Ferreri, 2007; Ghisalberti and Nepf, 2006) as a function of the Reynolds number.

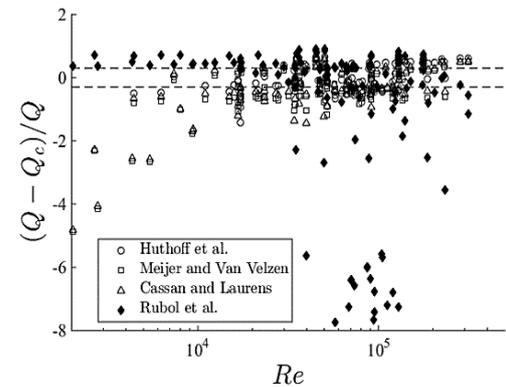


Fig. 7. Error between the 4 models and experiments from literature (Le Bouteiller and Venditti, 2015; Jarvela, 2005; Kouwen and Unny, 1969; Yang and Choi, 2009; Kubrak *et al.*, 2008; Huai *et al.*, 2009; Carollo, Ferro, Termini, 2002; Ciralo, Ferreri, 2007; Ghisalberti and Nepf, 2006) as a function of the Reynolds number.

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